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Geostatistical modeling of Rock Quality Designation (RQD) and geotechnical

2 zoning accounting for directional dependence and scale effect

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Abstract

The characterization of rock masses is an essential component for the planning and development of engineering designs in rock mechanics and rock engineering. The Rock Quality Designation (RQD) is a widely used rock mass characterization system that is direction-dependent, i.e., the measurement of a core sample depends not only on the sample position but also on its orientation. This paper outlines the critical aspects of the determination of RQD and proposes a physically-based upscaling strategy from borehole samples to large blocks, based on block-averaging the RQD values corresponding to the same direction, then calculating the minimum value over all the directions. An anisotropy index indicating how much RQD varies between one direction and another is also derived. Using geostatistical simulation, our proposal allows interpolating and upscaling direction-dependent geotechnical variables like RQD at any place in the geographical space for any

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direction, avoiding directional biases. We illustrate this proposal by predicting RQD in a polymetallic deposit, achieving geotechnical zoning and comparing the results with those of the traditional approach where the directional dependence of RQD is ignored.

Keywords: RQD; geostatistical simulation; geotechnical zoning; directional dependence;

upscaling.

1. Introduction

Proper zoning or domaining of areas presenting similarities in the lithological, structural, hydrogeological and rock quality components is of utmost importance for successful geotechnical designs in mining, geological and geotechnical engineering. The strength and deformability parameters of rock masses and the nature of the discontinuity network constitute complex information whose incorporation into the definition of different geotechnical domains is challenging and still a subject of significant uncertainties (Barton, 1990; Hudson, 2012; Chowdhury et al., 2012) that do not evade to the well-known RQD (Rock Quality Designation; Deere et al., 1967) classification system.

The RQD values are sensitive to the direction in which the core sample is collected (Palmström, 1982; Elsayed and Sen, 1991; Choi and Park, 2004; Emery and Séguret, 2020) and cannot be extrapolated straightforwardly to a more voluminous support, e.g., a three-dimensional block or the entire rock mass (Hoek and Brown,1980; Sen and Kazi, 1984; Cunha, 1990) without a proper management of the directional nature of the measurements. The directional dependence and the change of volumetric support (upscaling) are two critical aspects overlooked today by many practitioners, preventing a correct understanding of the spatial behavior of the rock mass and leading to inaccurate predictions.

The widely accepted practice of scaling the mechanical properties from a small piece of rock (e.g., a cylindrical borehole core, idealized as a 'line' support) to a three-dimensional 'block' support implies an assumption of isotropy (i.e., no directional dependence) of these properties at the working scale. Nevertheless, the geological materials often present spatial heterogeneities exhibiting a high contrast of the mechanical properties measured at two different locations, even at small distances (e.g., Cai, 2011; Song et al., 2011; Matonti et al., 2015; Pinheiro et al., 2016; Vatcher et al., 2016; Gao et al., 2018). Such a contrast may also vary with the relative angle between the measurements, being less when comparing two parallel borehole cores than when comparing two perpendicular cores. The integration of these subsurface variabilities into models to simulate the behavior of the rock masses would be theoretically the right way to go. Therefore, the predictive reliability of any model applied to rock engineering is strongly dependent on an accurate representation of the spatial and directional variability of the modeled variable(s). In this context, we propose to use a geostatistical model taking account of the directional sensitiveness of RQD to facilitate the upscaling from line-supports to block-supports and to obtain a better representation of the degree of jointing or fracturing of rock masses.

Geostatistical tools allow integrating spatial variability into the modeling of the rock mass. The application of geostatistics in geotechnics is not a new topic (e.g., Oh et al., 2004; Stavropoulou et al., 2007; Exadaktylos and Stavropoulou, 2008; Choi et al., 2009; Ferrari et al., 2014; Pinheiro et al., 2016; Chen et al., 2017; Hekmatnejad et al., 2017; Boyd et al., 2019). However, the traditional geostatistical modeling applied to RQD for characterizing rock masses (Ozturk and Nasuf, 2002; Ozturk and Simdi, 2014; Madani and Asghari, 2013) only considers its variability in the three-dimensional geographical space, disregarding its directional dependence, e.g., when there is a predominant joint set, such as foliation and schistosity joints in metamorphic rocks. This work aims to compare the traditional approach (ignoring directional dependence) with a new proposed approach considering the directional

dependence of RQD, illustrated with a case study of a polymetallic deposit. The results will highlight the advantages of the latter approach in the geotechnical zoning, knowledge of the spatial behavior of rock masses, and management of uncertainties in underground projects. The background and details of the proposed methodology are explained in Sections 2 and 3 and Appendix A, whereas the case study is presented in Section 4. A discussion, conclusions, and perspectives for future work follow in Sections 5 and 6.

2. Background: directional dependence and upscaling of RQD

The RQD rating (Deere et al., 1967) provides a quantitative measure of the degree of jointing or fracturing of rock mass from boreholes, consisting of 100 times the ratio between the total length of core pieces larger than 100 millimeters and the total core run length. The RQD classification system uses a continuous scale ranging from 0 to 100 to assign the rock mass quality and position it within one of five classes (excellent, good, fair, poor, very poor). In addition to the direct RQD calculation method, indirect methods have also been developed to estimate RQD considering different input data (Priest and Hudson, 1976; Palmström, 1982, 2005; Zheng et al., 2018) and to incorporate it into rock classification schemes (Bieniawski, 1973; Barton et al., 1974; Hoek et al., 2013). However, already since its conception, there is an awareness of one of its most obvious shortcomings: its directional dependence (e.g., Deere, 1989; Choi and Park, 2004). In the following, the directional dependence and upscaling will be described, providing details on how previous researchers have tackled them and how we will deal with them in our geostatistical model.

2.1 Directional dependence

The RQD values vary in space and according to the angle between the direction of the sample (borehole or scanline) and the discontinuities present in a rock mass. Since this directional dependence cannot be fixed, it is impossible to directly assess joint spacing conditions unless they do not depend on the direction (isotropic discontinuity network, whose properties are invariant under a rotation). Some attempts to minimize the biases caused by the directional dependence focused on drilling as many boreholes with different directions as possible (Deere, 1989), modifying the original RQD concept, such as estimating RQD from the volumetric joint count J_v (Palmström, 2005), considering Terzaghi's correction and a fractured zone effect (Haftani et al., 2016), or performing the calculation based on weak zones (core washed, crushed zones, karst cavities) and joint orientation (Azimian, 2016). However, an alternative definition of RQD is unknown to most engineers, practitioners, and researchers. Moreover, these new RQD conceptualizations lose their most potent and engaging property: simplicity. On the other hand, cost-efficiency and other practical considerations limit the number of boreholes drilled for an underground exploratory campaign.

An alternative solution to deal with RQD directional dependence has been proposed by Séguret and Guajardo (2015), who classified the borehole samples according to their sampling directions. However, this proposal does not circumvent all the limitations since RQD values can be predicted only in the directions that have been drilled, lacking any proper interpolation between them. Recently, Zheng et al. (2018) provide a new perspective about the directional dependence of RQD, considering this property as an advantage and proposing to estimate an anisotropy index of the jointing degree. The latter authors suggest selecting the minimum RQD value and its corresponding direction as the most representative of a specific rock mass, which potentially reflects its real jointing degree and directly compares with other rock masses. However, this approach only applies to scanlines (in outcrops or excavation faces) with different orientations for the same location, in relatively intact rock masses (from medium weathered to fresh hard), and statistically homogeneous regions. Except for the case of an isotropic discontinuity network, the selection of a

representative direction (here, the one corresponding to the minimum RQD) implies a loss of information on the directional behavior of RQD.

We build on the best of each previous attempt to propose an alternative solution that considers the real regionalization space of RQD as the usual geographical 3D space crossed by the 2D space of dip and azimuth. Specifically, the RQD for one composite sample depends on the geographical position \boldsymbol{x} (x_1 , y_1 , z_1) of its gravity center and the direction \boldsymbol{u} (α , ϕ) of the sample, with \boldsymbol{u} a point of the unit sphere \mathbb{S}^2 of \mathbb{R}^3 characterized by its azimuth α and dip ϕ , see Fig. 1 and Appendix A. This approach leads to the RQD measurements being assigned to five coordinates, which will allow evaluating the correlation between RQD values observed at different locations of this five-dimensional space (thus, depending not only on the geographical coordinates of the measurements but also on their angular coordinates).

2.2 Upscaling

The dependence of the mechanical properties of a rock mass with the geometric dimensions of the sample is known as the scale or support effect (Bieniawski, 1968; Pratt et al., 1972; Bandis et al., 1981; Barton, 1990; Cunha, 1990; Cuisiat and Haimson, 1992). This effect is a potential drawback for the spatial interpolation of mechanical properties observed at a limited (in size) sample to the overall rock mass. Considering that RQD in line supports (at borehole core or scanline scale) has a directional dependence, this dependence should remain in any subsequent change of support. In particular, such a change of support should not mix RQD values measured along different directions unless the discontinuity network in the rock mass is isotropic, in which case directional dependence does not arise.

In practice, the support effect can be overlooked when a particular rock mass consists of purely intact rock or individual jointed block pieces are too small compared to the overall

size of the engineering structure being considered. Hoek and Brown's criterion can thus be applied at the excavation/pit scale (Edelbro, 2004, Marinos and Carter, 2018). In such a situation, rock masses can be assumed as a continuum and isotropic medium, and the calculation of arithmetic averages from boreholes or scanlines (line-supports) to rock mass (block-supports) is justifiable (Marinos et al., 2005; Hoek, 2006). Then, when comparing the construction scale of rock excavations with the block size of intact rock, one can assume the rock mass is closely jointed and be treated as a homogeneous continuous equivalent whose discontinuities are implicit. However, such an assumption is not always possible and rarely occurs due to the nature of geological materials affected by tectonism, weathering, and alteration processes.

Our approach to addressing the support effect is twofold. On the one hand, it is proposed to define a direction-dependent upscaled RQD value in a given block by averaging the RQD values measured along the same direction at different points discretizing the block, without mixing values measured in different directions. In this way, it is possible to know the directional variability of RQD in each block, accounting for the anisotropy of the rock mass and giving insights into the geometry of the fragments formed by the intersection of joints in a rock mass. On the other hand, in addition to this 'directional' block-support RQD, we will also propose a 'non-directional' block-support RQD by selecting the minimum RQD value (not the average) across all the directions of the two-dimensional angular space represented by a unit sphere, which is deemed the most representative value of the real jointing degree of the block. Accordingly, the 'directional' block-support RQD will be regionalized in the five-dimensional (geographical × angular) space, as is the RQD measured on core samples, while the 'non-directional' one will be regionalized only in the 3D geographical space.

3. Methodology: geostatistical modeling and simulation

Since it is defined on a continuous quantitative scale, RQD can be transformed into a variable (hereafter, denoted as Z) with a standard Gaussian distribution, a process known as Gaussian anamorphosis or normal-scores transformation (Chilès and Delfiner, 2012). Such a transformation is the first stage for geostatistical modeling, where the transformed variable (Z) is viewed as a Gaussian random field. In practice, the characterization of such a random field reduces to that of its mean value (here, the mean is set to zero) and its autocovariance function or, equivalently, its variogram.

- To simulate RQD, the following steps should be accomplished (details in Appendix 193 A):
 - I. The original RQD data are transformed into Gaussian data (normal scores), and an anamorphosis function that maps the RQD data into normal scores is defined.
 - II. A variogram analysis of the normal scores data is performed, consisting in computing an experimental variogram that measures half the variance of the increment between two measurements based on their geographical and angular separations when RQD is regionalized in a 5D space or just on their geographical separation when RQD is regionalized in the 3D space. Subsequently, the experimental variogram is fitted with a theoretical model.
 - III. A Gaussian random field is simulated at all the target locations and conditioned to the available data. The latter implies that the simulated values at locations with data must coincide with the data values. Here, we use the turning bands algorithm (Matheron, 1973) for the random field simulation and a post-processing kriging (Chilès and Delfiner, 2012) for conditioning the simulated outcomes to the sampling data.
 - IV. The Gaussian simulation is back-transformed to the RQD original scale by using the anamorphosis function defined at step I.

4. Case study: jointed rock mass in a polymetallic deposit

To illustrate the proposed methodology and compare it with the traditional approach, we present a case study corresponding to an underground mining operation. The aims are to simulate RQD within the deposit, considering its directional dependence and a change of support that can adequately characterize a volumetric support and be used for geotechnical zoning.

4.1. Geological setting and data preparation

The case study corresponds to a polymetallic deposit, the name and location of which will not be disclosed for confidentiality reasons. In an irregular tubular body, the mineralization is of the distal skarn type, related to intrusives of intermediate composition, where the most frequent alterations are chloritization and skarn type. The ore zone mainly comprises skarn and limestones.

The deposit is located along a deformed belt composed of back-arc siliciclastic and carbonates rocks, unconformably covered with volcanic rocks, intruded by intermediate granodiorite and quartz-monzonite stocks and sills, the emplacement of which is structurally controlled by north-to-south-oriented faults. The structural evolution involves compressional reactivation of pre-existing extensional faults and strike-slip episodes of deformation. Two main structural domains have been identified: an ENE-vergent fold-and-thrust system with steep dip angles (75° SE), while the second one is linked to dextral strike-slip movements with variable strikes and steep dips angles (60-80° SE). The discontinuities identified from geotechnical boreholes can be grouped into three main families, two sub-vertical and one sub-horizontal, the orientations of which corroborate the influence of the regional structural

background at the ore deposit scale rock mass (Fig. 2). Locally, sub-vertical and mid-dip secondary discontinuity systems also arise.

The rock mass is stratified with a moderate weathering, the spacing of the strata being between 6 and 20 cm, with a persistence greater than 20 m, opening less than 1 mm without filling, except for a slime patina. The metamorphosed limestone rock mass is more competent than rocks of similar composition, but is more brittle, with a higher Hoek-Brown modulus value (mi = 14) compared with their unmetamorphosed parent carbonate rocks (mi = 12). The ore zone and its environment have rock mass qualities that generally range from fair to good (RMR between 50 and 75), except for localized fault zones where the rock mass quality is poor (RMR between 25 and 35). The water condition is humid and adversely influences the stability of the underground excavations, and can vary to wet due to water seepage in fault areas.

Boreholes have been drilled from the surface and from underground galleries, along which geologists have measured RQD and other geotechnical variables on intervals of lengths varying between 1 and 4 m. To model RQD, we calculated average values for 3 m long composites along the boreholes. The resulting database contains 3800 composited data with their locations (easting, northing, elevation, azimuth, dip) and rock quality designations. The sampled volume is about $350 \times 500 \times 700$ m³.

4.2. RQD modeling

As the distribution of RQD differs significantly from a Gaussian distribution (Fig. 3), a normal score transformation (anamorphosis) is performed before variogram analysis and simulation. The transformation accounts for declustering weights calculated with the cell

method (Journel, 1983), giving more importance to isolated data and downweighting clustered data to correct the effects caused by the irregularities of the sampling mesh.

We performed variogram calculations according to the parameters listed in Table 1 to identify preferential directions of continuity in the geographical space. The calculated variograms (Fig. 4) show that no significant anisotropy in the geographical space exists for the normal scores data, be they regionalized in the 3D or the 5D space. The latter owes to experimental variograms calculated along different directions of the 3D space (with a fixed angular separation between paired data in the 5D approach) overlap to a great extent. Thus, henceforth, only omnidirectional variograms are calculated in the geographical space (Fig. 5). The discontinuity observed at the origin ('nugget effect') is interpreted as a consequence of the small-scale variability of RQD, where continuity is not perceptible. On the other hand, it is also seen that the experimental variogram increases with the geographical separation distance and with the angular separation of the paired data until it reaches a sill at geographical separations of about 60 to 100 m. The better continuity of RQD (slower increase and lower sill) occurs when the angular separation between the measurements is low.

These experimental variograms of the 5D regionalized data are fitted by basic nesting models, each being the product of two components: a stationary geographical correlation and an isotropic angular correlation. The fitted variogram model is the following:

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$$\gamma_{RQD}(\mathbf{h}, \delta) = (0.19)nugget(\mathbf{h}) + (0.20)sph_{20,0}(\mathbf{h}, \delta) + (0.35)sph_{40,0}(\mathbf{h}, \delta)$$

$$+ (0.14)sph_{130,0}(\mathbf{h}, \delta) + (0.10)sph_{130,2}(\mathbf{h}, \delta) + (0.14)sph_{\infty 2}(\mathbf{h}, \delta)$$

with h and δ being the geographical and angular separations between two measurements (Fig. 4e), and $sphL_{a,0}(h,\delta)$ being one minus the product of an isotropic spherical correlation

with a range a evaluated at h, and a Legendre polynomial of degree n evaluated at $\cos(\delta)$, see Eq. (A.4) in Appendix A.

The variogram model for the 5D regionalized data fits quite well the calculated experimental points for all the different geographical and angular separations between paired data (Fig. 5). The first milestone on the ordinate axis corresponds to the nugget effect, with a partial sill of 0.19. After that, one uses three nested structures corresponding to the product of a spherical covariance with a range of 20, 40, or 130 m and a zero-degree Legendre polynomial. The latter is identically equal to 1, so that the three nested structures exclusively depend on the geographical separation. Finally, two nested structures consider the product of long-range spherical covariances with a second-degree Legendre polynomial, varying with the angular separation. The use of a zonal anisotropy (spherical structure with an infinite range) and the fact that the second-degree Legendre polynomial changes from positive to negative values as the angular separation increases allow modeling the increase in the variogram sill with the angular separation.

For the traditional approach (3D regionalized data), we consider only the basic nested structures that depend on the geographical coordinates, and the variogram model remains as follows:

$$\gamma_{ROD}(\mathbf{h}) = (0.19) nugget(\mathbf{h}) + (0.20) sph_{20.0}(\mathbf{h}) + (0.35) sph_{40.0}(\mathbf{h}) + (0.24) sph_{130.0}(\mathbf{h})$$

The proposed variogram models (for both the traditional and directional approaches) are validated using leave-one-out cross-validation techniques (Fig. 6). The RQD at each data location is simulated 500 times conditionally to the neighboring data in a radius of 200 m, excluding the five adjacent composites from either side of the same borehole to avoid considering data too close to the target location. The outcomes of the 500 simulations are

then averaged to obtain a prediction at the data location, which is compared against the true RQD value. The dispersion diagram between predicted and true RQD values for both approaches has a regression line that matches the first bisector (Figs. 6a, c), proving that the simulations are conditionally unbiased (Chilès and Delfiner, 2012). Furthermore, accuracy plots (Goovaerts, 2001) allow evaluating the capability of the 500 simulations to measure the uncertainty associated with the true values: the fraction of true values belonging to the p-probability interval is practically equal to p, whatever this probability is in [0,1] (Figs. 6b, d).

4.3. Conditional simulation

Simulation is performed in the geographical space on a regular grid with a mesh of 2 m x 2 m x 20 m covering part of the sampled region, for three directions in the angular space: north, east and vertical. Five hundred simulations of RQD are constructed at each target grid node and direction. The maps plotted in Figs. 7 and 8 show a horizontal slice (43,750 nodes) at elevation 350 m above the mean sea level of the geographical space. The borehole data distant less than 10 m from the target grid (Fig. 7) or less than 10 m from the target grid and 45° from the target direction (Fig. 8) are superimposed, together with envelopes delimitating 'confidence regions' inside which the kriging error variance is less than 90% of the data variance (i.e., the borehole data are informative and significantly reduce the uncertainty inside the envelope). In the maps of the average of the 500 simulations, the most remarkable contrast in RQD is observed in areas where the borehole data are present for both the traditional (Fig. 7b) and directional (Figs. 8b, d, f) approaches. This contrast is consistent with field information, according to which the rock mass quality is good (RQD > 75) in the eastern side of the sampled area, corresponding to the ore zone, and becomes regular (RQD < 50) in the western side.

For both approaches, the simulated RQD values vary from a poor (RQD ~40%) to an excellent (RQD >90%) rock quality in just a few tens of meters apart (Figs. 7a, 8a, 8c, 8e). The map for the average of the 500 simulations using the traditional approach (Fig. 7b) shows a more significant similarity with the average map of the directional approach when RQD is simulated along the north direction (Fig. 8b). This coincidence is an artifact since most of the boreholes in the study area are oriented sub-horizontally and mainly in a north-southeast direction, as corroborated by the superimposed borehole data on the maps.

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Geologists and mining engineers could interpret very favorable conditions towards the east of the study area only based on Fig. 7b (traditional approach), corroborating the influence of the major structural domains (ENE-vergent fold-and-thrust system with dip angle 75° SE, and dextral strike-slip movements with dips angles 60-80° SE). Even though it is true that a better rock quality is present in that sub-area, this interpretation is conditioned by the RQD values measured along a specific sampling direction, which is likely to be biased with respect to the RQD measured in other directions for the same area. The risk of bias is minimized when RQD is simulated along different directions using the directional approach, and one can be aware of the variations of RQD in the geographical space and the angular space. The good rock quality towards the east, evidenced in all the maps, above all along the vertical and north directions (Figs. 8b, d, f), can be confidently interpreted as the real behavior of the rock mass. The comparison between the simulation results in the absence or presence of a directional component allows visualizing the spatial variability to be expected in the field for a given direction. The latter is helpful in quantitatively define favorable conditions for the advance of the excavation from a quantitatively and non-qualitatively point of view, as is currently done in Bieniaswski's RMR classification.

4.4. Change of support (upscaling)

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The change from the sample support (cylindrical borehole composite) to block support implies averaging simulated RQD values in the geographical space. However, it does not make sense to average RQD values associated with different directions. For the change of support to be meaningful, a single direction has to be chosen, and all the RQD values being averaged should correspond to this direction. In the traditional approach, the selection of an appropriate direction to conduct a change of support is a challenge, insofar as it is necessary to find a 'representative' direction for (a) measuring RQD and (b) averaging the RQD values on the block: otherwise, one calculates a non-directional block-support RQD value that mixes different directions (Fig. 9, right panel). In the proposed (directional) approach, the problem is solved straightforwardly by defining as many block-support RQD as directions of interest, with no need for the data to be measured along the same direction because RQD can be simulated for each geographical coordinate and each direction (Fig. 9, top left). The regionalization in a 5D space (left panel) therefore allows calculating a directional RQD. All the calculations can be made on each simulation separately or averaged over the 500 simulations to obtain a prediction.

If a block has to be characterized by a single RQD value, the minimum RQD over all the directions should be considered, which leads to a 'non-directional' block-support RQD (Fig. 9, middle left). In such a case, the change of support is essentially non-additive as it relies on a minimum and not an average over the directions. Since the use of a single RQD value to represent the degree of jointing or fracturing in a block is less informative than a direction-dependent RQD (given the high variability in the angular space), we propose to complement the non-directional (minimum) RQD with an anisotropy index (AI) of jointing degree for rock masses, following Zhen et al. (2018). The AI measures the spread or dispersion of the RQD for each block within the angular space. In this work, the AI of the jointing degree is defined for each block and each simulation as one hundred times the

difference between the maximum and minimum RQD values across all the directions, divided by the maximum RQD value (Fig. 9, bottom left):

$$AI = 100 \left(\frac{RQD_{max} - RQD_{min}}{RQD_{max}} \right).$$

Figure 10 shows that the anisotropy index is between 30% to 80%, with a marked contrast in the eastern part of the map close to the sampled area. This anisotropy of the jointing degree for the rock mass (directional variability of RQD) is not negligible and cannot be detected when RQD is regionalized in the three-dimensional geographical space and the directional component is discarded (i.e., using the traditional approach).

In addition to the minimal RQD over all the directions (as the representative value of a block) and the anisotropy index (indicating how much RQD varies from one direction to another), one can identify the direction for which the minimal RQD is reached. This direction sheds light on the anisotropy of the discontinuity network in the rock mass and on the existence of preferential fracturing directions, as in the Terzaghi concept (Zhen et al., 2018) (the minimum RQD is expected to occur along the direction perpendicular to the fracture planes). Such an analysis can be made locally (for a single block) or for a group of blocks. As an illustration, Fig. 11 is an azimuthal projection of the upper hemisphere of an equal-angle polar net (Priest, 1985) showing the directional concentration for the minimum simulated RQD at a regular grid of the geographical space (1,750 blocks) and 100 directions spanning the space at intervals of 36° and 18° in azimuth and dip, respectively, where the gray dots represent the sampling directions. On average, the RQD rating decreases by 20.66% when considering the direction of the minimum value per block due to its directional dependence. The minimum RQD is more frequently reached along directions of azimuth between 330° to 350° and dip between 30° and 45°, or directions with azimuth between 285° and 315° and

dip between 30° and 45°. The latter suggests that the fracture planes tend to be sub-horizontal to oblique with a northwest-north direction, similar to the orientation of the parallel structural domain (NW-SE).

RQD can be predicted at the block support by averaging the simulated RQD over many scenarios, as shown in Figs. 7 and 8 for the composite support. When the results of the 3D traditional approach (Fig. 12a) are compared with those using the directional approach (Fig. 12b-f), several differences arise, highlighting the influence of the directional dependence of RQD in the geomechanical zoning of the rock mass. In essence, Fig. 12a bears more resemblance to Fig. 12e, where RQD is simulated along the north direction, than to Fig. 12e and 12f corresponding to RQD simulated along the east and vertical directions. This reveals a bias of the RQD predicted with the traditional approach, conditioned to particular sampling directions, and cannot be extrapolated to other directions. In contrast, the directional block-support RQD (Fig. 12d-f) is more informative and helpful for geotechnical designs and rock mass rating or geotechnical zoning. Since it is an unbiased representation for a specific direction, it can evaluate the impact of the advance of the rock excavation in this direction in rock mass mechanical behavior.

To exemplify the impact of directional RQD modeling in engineering decision-making, consider a tunnel bored along the north-south direction with a width of 30 ft (9.4 m) in an igneous/metamorphic rock, typical in the study area, where real rock pressures or swelling/squeezing ground do not exist. According to the rock support classification system based on RQD for tunnels of varying widths (Deere, 1989), either steel sets or reinforced shotcrete or RIB is compulsory almost everywhere if one relies on the RQD corresponding to the worst-case scenario for each block (Fig 12c), which may be too pessimistic. In contrast, based on the directional block-support RQD associated with the tunnel direction (north-south) (Fig 12d), only a pattern bolting (4-6 ft centers) or a 4-6 cm shotcrete is required in the

eastern side of the area under study. On the other hand, considering the RQD models obtained with the traditional approach (Fig. 12a) or with the average scenario (Fig 12b) (average RQD over all the directions and all the simulations) gives a misleading representation of the actual rock quality in the eastern part, with a minimal requirement (no support or local bolts, which is too optimistic).

4.5. Geotechnical zoning

The lithological characteristics and the degree of alteration are homogeneous in the studied area; therefore, the rock mass quality is the most relevant criterion for its geotechnical zoning. Our results on the upscaled RQD may conveniently be integrated into widely used rock mass classification systems, such as the RMR and its modifications (Bieniawski, 1973, 1989; Hoek et al., 2013; Bertuzzi et al. 2016), the Tunneling Quality Index (Q; Barton et al., 1974) and the Geological Strenght Index (GSI; Marinos and Carter, 2018). Following Deere et al. (1967), we relate our simulated block-averaged RQD value with the engineering rock mass quality and implement a geotechnical zoning map (Fig. 13). For each block and each simulation, the simulated block-support RQD is assigned one of five classes (excellent, good, fair, poor, very poor), then the class that most frequently appears across the 500 simulations is retained as the final classification of the block.

The zoning maps so obtained strongly differ, depending on whether one considers the non-directional RQD calculated with the traditional approach (Fig. 13a), or the average (Fig. 13b) or the minimum (Fig. 13c) RQD over all the directions calculated with the proposed approach. The former approach ignores the directional dependence of RQD and mixes measurements made in different drilling directions. Therefore, the map in Fig. 13a lacks physical sense, where the primary class is a fair quality rock, with 85% of the blocks, followed by a good quality rock with 8%. The remaining are between poor (3.7%) and

excellent (3.3%). A similar mixing arises with the map in Fig. 13b: although RQD is regionalized in a 5D space, the simulated values are then averaged over all the directions, yielding zoning similar to the traditional approach (poor 0.3%, fair 88.1%, good 11.5%, and excellent 0.1%). In contrast, the map in Fig. 13c only considers the 'worst' direction for each block (the one associated with the lowest RQD) and yields a more conservative definition of the geotechnical domains, where almost three-quarters of the blocks are classified as poor to fair rock (73.4%) and the rest as very poor. This is the price to pay to get geotechnical zoning that is non-directional and, at the same time, physically meaningful.

5. Discussion

Contribution of our work. Like all types of one-dimensional measurements (borehole cores and scanlines), RQD depends not only on the geographical position of the sample but also on its direction, precluding a direct averaging that mixes different directions as a proper upscaling strategy. Upscaling must appropriately address the directional dependence to reduce uncertainties in geotechnical projects. Our proposal considers regionalizing the RQD data in a five-dimensional space corresponding to the three-dimensional geographical space crossed with the two-dimensional sphere, allowing RQD to be interpolated at any place in the geographical space and for any direction. Knowing the rock mass quality in specific directions is beneficial to evaluate the impact of the discontinuity orientations for tunnels, slopes, or foundation designs. The latter becomes critical, bearing in mind that the rock mass behavior is influenced by the regional geological structures rather than by the strength of intact rock, as shown, for instance, by the coincidence of the orientation of minimum RQD values (Fig. 11) with regional structural faults.

Applicability. The 5D geostatistical approach requires sampling data with information on RQD and on the borehole positions and orientations, information that should always be

logged and readily available in any good geotechnical database. The spatial and directional behaviors of RQD can be modeled in a flexible manner, using basic nested structures to fit the experimental variogram of the RQD data. This makes the proposal applicable to an extensive range of rock masses and conditions, including both weak and complex situations, such as tectonized (disturbed and broken by structural dislocation, shearing, folding, or compression) or heterogeneous (flysch formations or molassic formations) rock masses.

Practical limitations. If all the boreholes have the same or almost the same orientation (e.g., vertical), then it can be challenging to infer the directional behavior of RQD and to apply the 5D geostatistics approach. Other practical limitations of this approach are the assumptions of stationarity in the geographical space and isotropy on the sphere (see Appendix). The former assumption is often sensible after partitioning the deposit into areas ('geotechnical domains') with similar structural and mechanical characteristics, and performing the geostatistical analysis within each domain separately. As for the latter assumption of isotropy, it may be questionable when there is only one network of parallel fractures, in which case it would make sense to work with anisotropic variogram models on the sphere.

3D vs. 5D modeling. The traditional (three-dimensional, non-directional) and proposed (five-dimensional, directional) approaches to modeling RQD in rock engineering applications have been cross-validated, providing a good fit in terms of prediction and uncertainty assessment. Each approach has its pros and cons. For lower computational requirements and pre-processing time, the traditional approach achieves a globally good prediction of RQD but is locally biased as per the drilling direction. The directional approach is more demanding in terms of modeling and computational capacity but provides information on the directional RQD behavior, which is valuable for geotechnical zoning and decision-making as it reflects the inherent nature of the geotechnical parameters (their directional

dependence) and can give an insight into the geometry of the rock fragments when combined with other direction-dependent parameters such as the fracture frequency. This approach accounts for the fact that, in the presented case study, one measurement of RQD provides much information about values at surrounding locations along the same or nearly parallel directions, as indicated by low variogram values at short separation distances and small separation angles (Fig. 5). However, an RQD measurement provides less information in a perpendicular direction. Therefore, it constitutes good practice to account for the directional dependence of RQD in any geostatistical modeling and upscaling analysis.

6. Conclusions and perspectives

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The common practice in geotechnical modeling overlooks the directional dependence of geotechnical variables and characterizes volumetric support assuming rock masses as a continuum and isotropic medium and extrapolating or averaging sample-support information (from boreholes or scanlines) to a three-dimensional block support or to the rock mass. However, rock masses are seldom isotropic and are generally heterogeneous, hence our proposal to tackle the change of support problem by accounting for the inherent directional dependence of RQD (5D regionalization) and for its uncertainty at unsampled locations (use of geostatistical simulations). This change of support is performed by averaging the RQD simulated along a specific direction on a grid, discretizing each block in the geographical space. The upscaled directional RQD is practical, simple, and does not modify the original concept of RQD, making it suitable and serviceable in engineering applications, e.g., to determine required tunnel support. The directional approach better reproduces the geographical and directional heterogeneity and real nature of the rock mass. A nondirectional upscaled RQD can also be derived from this approach by considering the minimum value obtained over all the directions (corresponding to the most unfavorable direction of fracturing, i.e., the worst case scenario) and complemented with an anisotropy index of jointing degree (sensu Zheng et al., 2018). Both the directional and non-directional RQD so obtained directly impact the prediction of safety factors and control measures in rock engineering projects.

Our directional approach can be used for geotechnical zoning (Fig. 13), i.e., classifying the rock mass into similar design areas. Besides helping determine fault zones, and by applying empirical formulas, it is also possible to calculate other mechanical parameters such as the modulus of elasticity or the unconfined compressive strength (Zhang and Einstein, 2004, Zhang, 2016). This approach is helpful in any stage of a geotechnical project. At the exploratory or early stages, it is suggested to use adaptive geometries in the block model: the size of the block will depend on the variability of the geotechnical parameter in the sector and the available information, i.e., the less amount of available information (and the more variability of RQD), the larger the block size. At the development and production stages, it is possible to model the geotechnical parameters, whether in a parallel or perpendicular direction, to advance the construction of the excavation in rock.

One way to optimize the analysis is to use adaptive geometries in the block model in further developments, giving a higher resolution (smaller block size) in areas with a higher anisotropy index or vice versa. Moreover, for upscaling directional variables such as RQD, it is interesting to analyze the relationship between strength and stiffness versus block size to determine the Representative Volume Element size, REV (Zhang et al., 2017).

To broaden the scope of application of the presented proposal, future works in the 5D geostatistical modeling should include the design of variogram models and simulation algorithms using non-separable covariance functions or anisotropic covariances on the sphere, together with exploratory tools to identify preferred directions of anisotropy on the sphere based on sampling information.

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Appendix A

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A.1. Traditional approach: modeling RQD in the 3D Euclidean space

This approach considers that RQD varies only with the geographical coordinates (easting, northing, and elevation), i.e., regionalized in the Euclidean space \mathbb{R}^3 . A common practice for geostatistical modeling is to assume second-order stationarity, i.e., the mean value and the covariance function or the variogram are invariant under a translation in space, which allows their inference from a set of sampling data (Chilès and Delfiner, 2012). A convenient way to model the experimental covariance (or experimental variogram) is through a positive linear combination of basic nested structures:

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$$C(\mathbf{h}) = cov\{Z(\mathbf{x} + \mathbf{h}), Z(\mathbf{x})\} = \sum_{s=1}^{Smax} b_s \rho_s(\mathbf{h})$$
 (A.1)

where Z is the Gaussian random field associated with RQD, \mathbf{x} and $\mathbf{x}+\mathbf{h}$ are two points in the geographical space separated by vector \mathbf{h} and, for $\mathbf{s}=1,...$ S_{max} , b_s is a nonnegative real value, and ρ_s is an autocorrelation function (positive semi-definite function taking the value 1 at $\mathbf{h}=\mathbf{0}$).

The Gaussian random field can then be simulated as a sum of S_{max} components, each associated with a particular nested structure:

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$$Z^{(S)}(x) = \sum_{s=1}^{Smax} \sqrt{b_s} Z_s^{(S)}(x)$$
 (A.2)

where the superscript (S) stands for 'simulated'. The reader is referred to Emery and Lantuéjoul (2006) for algorithmic details on the simulation process for the nested structures commonly used in geostatistical applications.

A.2. Directional approach: modeling in a 5D product space

To account for the fact that RQD is direction-dependent, the associated random field Z is now defined in a five-dimensional space ($\mathbb{R}^3 \times \mathbb{S}^2$, with \mathbb{S}^2 the unit sphere), i.e., Z =

 $\{Z(\mathbf{x},\mathbf{u}): \mathbf{x} \in \mathbb{R}^3 \text{ and } \mathbf{u} \in \mathbb{S}^2\}$, where \mathbf{x} represents the geographical coordinates of the measurement, and \mathbf{u} the direction (azimuth and dip) of this measurement.

The simplest way to model the spatial correlation of regionalized data in such a 5D space is to consider second-order stationarity in the geographical space and second-order isotropy on the sphere. These assumptions imply that the mean value is constant and that the covariance function or the variogram between the two random variables located at (x,u) and (x',u') in $\mathbb{R}^3 \times \mathbb{S}^2$ only depends on the separation vector h = x' - x and on the geodesic distance or angular separation $\delta(u,u') = \arccos(\langle u,u' \rangle)$, with $\langle \cdot, \cdot \rangle$ the inner product. The modeling can be extended as follows:

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$$C(\mathbf{h}, \delta) = \sum_{s=1}^{S_{max}} b_s C_s(\mathbf{h}, \delta)$$
 (A.3)

where, $b_s \ge 0$ for s = 1,... S_{max} and C_s is a basic autocorrelation (positive semi-definite) function defined on $\mathbb{R}^3 \times [0, \pi]$. In this work, separable basic autocorrelation functions are used:

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$$C_{s}(\boldsymbol{h},\delta) = \rho_{s}(\boldsymbol{h})P_{n(s)}(\cos\delta) \tag{A.4}$$

where ρ_s is an autocorrelation function in \mathbb{R}^3 and P_n the Legendre polynomial of degree n. Schoenberg (1942) showed that the mapping $\delta \mapsto P_n(\cos \delta)$ is an isotropic correlation function on the sphere. Because the Legendre polynomial P_n has the same parity as n, and because the RQD measurement along a direction \boldsymbol{u} is the same as along the opposite direction $-\boldsymbol{u}$, the covariance should remain the same when changing $\delta(\boldsymbol{u}, \boldsymbol{u}')$ into π - $\delta(\boldsymbol{u}, \boldsymbol{u}')$, i.e., when changing $P_n(\cos \delta)$ into $P_n(-\cos \delta)$. Accordingly, only even degrees n(s) should be considered in Eq. (A.4).

As for the previous approach, the Gaussian random field can be simulated as a sum of S_{max} components. Each component is associated with a particular nested structure and separates into the product of a geographical component and a directional component (Sanchez et al., 2019):

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$$Z^{(S)}(\mathbf{x}, \mathbf{u}) = \sum_{s=1}^{Smax} \sqrt{b_s} Z_s^{(S)}(\mathbf{x}) W_s^{(S)}(\mathbf{u})$$
 (A.5)

with $Z_s^{(S)}(x)$ a zero-mean random field in the Euclidean space with autocorrelation $\rho_s(\mathbf{h})$, and $W_s^{(S)}(\mathbf{u})$ a zero-mean random field on the sphere with autocorrelation $P_{n(s)}(\cos\delta)$, see Emery and Porcu (2019) or Lantuéjoul et al. (2019) for examples on how to simulate random fields on the sphere.

811 Tables

	Horizontal variograms	Vertical variograms
Azimuth (°)	0, 90, 0	0, 0, 0
Dip (°)	0, 0, 45	90, 0, 135
Lag separation (m)	10	10
Number of lags	45	45
Angular separation between data (°)	0, 30, 60, 90	0, 30, 60, 90
Azimuth tolerance (°)	90, 20, 90	90, 20, 90
Dip tolerance (°)	20	20, 90, 20
Lag tolerance (m)	5.0	5.0
Angular separation tolerance (°)	15	15

Table 1. Parameters for experimental variogram calculations

Figure captions 817 818 819 Figure 1. Geographical and angular spaces. Each core sample (blue cylinder) is indexed by the easting, northing 820 and vertical coordinates of its gravity center in the geographical space, as well as its azimuth and dip in the 821 angular space, totaling 5 coordinates. The measured RQD values depend on both the geographical and angular 822 coordinates. 823 824 Figure 2. Block modeling of rock mass class (based on rock quality designation observed at 3800 core samples) 825 in a volume of 350 m along the east direction, 500 m along the north direction, and 700 m along the vertical 826 direction (polymetallic deposit). The classes can be associated with lithological and structural characteristics of 827 the deposit. 828 829 Figure 3. Experimental histogram of Rock Quality Designation (RQD) of borehole data composited at a length of 830 3 m. 831 832 Figure 4. Experimental variograms of normal scores data in horizontal, vertical, and oblique directions, for 833 geographical separations ranging from 0 to 500 m and angular separations equal to (a) 0°, (b) 30°, (c) 60°, and 834 (d) 90°. The geographical separation measures the distance between the gravity centers of the paired samples, 835 while the angular separation measures the difference between their orientations (e). 836 837 Figure 5. Experimental (asterisks) and modeled (solid lines) variograms of the normal scores data for 838 geographical separation distances ranging from 0 to 150 m and angular separations between paired data ranging 839 from 0° to 90°. 840 841 Figure 6. Leave-one-out cross-validation results: (a, c) scatter plots between true RQD (vertical axis) at the 842 sampling locations and the average of 500 simulations (horizontal axis) conditioned to the data in a neighborhood 843 of the target location (excluding the five nearest composites on either side of the same borehole), and (b, d)

accuracy plots showing the proportion of data belonging to a probability interval as a function of the interval

probability, (a, b) for traditional and (c, d) directional approaches.

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Figure 7. Simulation of RQD using the traditional approach (ignoring directional dependence and regionalizing RQD in the 3D geographical space only). (a) Map of the first simulation. (b) Map of the average of 500 simulations. Black dots correspond to the borehole data distant less than 10 m from the grid, and the contour represents the envelope of the kriging variance equal to 0.9 times the data variance.

Figure 8. Simulation of RQD using the directional approach (regionalizing RQD in the 3D geographical space crossed with the 2D angular space). (a, c, e) Maps of the first simulation for (a) north, (c) east, and (e) vertical target directions. (b, d, f) Maps of the average by direction of 500 simulations for (b) north, (d) east, and (f) vertical target directions. Black dots correspond to the borehole data distant less than 10 m from the grid and 45° from the target direction. The contour represents the envelope of the kriging variance equal to 0.9 times the data variance.

Figure 9. A synthesis of the different upscaling proposals. Ns stands for the number of samples per block, Nb for the number of blocks in the geographical space, Nk for the number of simulations, and Nj for the number of directions in the angular space.

Figure 10. Map of anisotropy index (AI) of jointing degree using the directional approach (average index over 500 simulations). The blue contour represents the envelope inside which the kriging variance in all the directions is less than 0.9 times the data variance (intersection of the directional envelopes as defined in Fig. 8). The black contour represents the envelope inside which the kriging variance in at least one direction is less than 0.9 times the data variance (union of the directional envelopes as defined in Fig. 8). Both envelopes enclose 'confidence regions' in which the borehole data are informative.

Figure 11. Upper hemispherical equal angle polar projection net or regionalized azimuthal projection showing the concentration of the directions for which the minimum block-support RQD is reached. Calculations consider 500 simulations and 1,750 blocks of $10 \times 10 \times 20$ m in a horizontal section of the geographical space. The sphere is discretized into 100 directions. Concentric circles represent the dip/plunge each 30°, increasing from outside and azimuths measured clockwise in degrees from north indicated in the out end the projection. The grey dots correspond to the sampling directions of the borehole data.

Figure 12. Map of the average of 500 simulations of block-support RQD obtained with the (a) 3D traditional approach and with the directional approach; (b) average block-support RQD over all the directions; (c) minimum block-support RQD over all the directions; directional block-support RQD along the (d) north, (e) east and (f)

vertical directions. The maps in (a) and (b) mix different directions and do not have a clear physical meaning, while the maps in (c-f) only refer to a single direction per block (most conservative direction in (c), which may vary from block to block, and fixed direction in the other maps).

Figure 13. Geotechnical zoning map using block-support (upscaled) RQD. (a) The most probable class is based on 500 simulations obtained with the traditional 3D approach. (b) and (c) Most probable class based on 500 simulations obtained with the 5D directional approach: average over all the directions (b) and minimum RQD over all the directions (c).

























